STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH JOB SPECIFICATIONS AND ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, AND SITE DRAWINGS. CONSULT THESE DRAWINGS FOR SLEEVES, DEPRESSIONS, AND OTHER DETAILS NOT SHOWN ON STRUCTURAL DRAWINGS.

CONTRACTOR SHALL LOCATE ALL BURIED UTILITIES PRIOR TO EXCAVATION FOR BUILDING FOUNDATIONS. THE STRUCTURAL ENGINEER SHALL BE NOTIFIED OF POTENTIAL CONFLICTS BETWEEN FOUNDATIONS AND BURIED UTILITIES.

CONTRACTOR SHALL VERIFY EXISTING CONDITIONS OF THE SITE. THE STRUCTURAL ENGINEER SHALL BE NOTIFIED OF POTENTIAL CONFLICTS OR DISCREPANCIES PRIOR TO PROCEEDING WITH CONSTRUCTION.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROPER LOCATION AND SIZE OF OPENINGS FOR ALL TRADES AND SHALL COORDINATE ALL CONSTRUCTION AS INDICATED BY THE CONSTRUCTION DOCUMENTS, INCLUDING SHOP DRAWINGS REVISED BY THE STRUCTURAL ENGINEER.

NO PENETRATIONS SHALL BE MADE IN ANY STRUCTURAL MEMBERS LOCATED ON THESE DRAWINGS WITHOUT PRIOR APPROVAL OF THE ENGINEER, UNLESS NOTED OTHERWISE.

DO NOT EMBED HORIZONTAL - CONDUITS, PIPES, ETC., IN HORIZONTAL CONCRETE STRUCTURAL MEMBERS UNLESS SPECIFICALLY SHOWN ON THE STRUCTURAL DRAWINGS.

DO NOT EMBED VERTICAL CONDUITS, PIPES, ETC., IN VERTICAL CONCRETE STRUCTURAL MEMBERS, UNLESS SPECIFICALLY SHOW ON THE STRUCTURAL DRAWINGS.

CENTER ALL FOOTINGS AND PIERS UNDER COLUMNS ABOVE UNLESS SPECIFICALLY DIMENSIONED OTHERWISE.

CODE REQUIREMENTS: THE BUILDING STRUCTURE IS DESIGNED IN ACCORDANCE WITH THE 2023 8TH EDITION OF THE FLORIDA BUILDING CODE. FOLLOW ALL APPLICABLE PROVISIONS FOR ALL PHASES OF CONSTRUCTION. ADDITIONS ARE IN COMPLIANCE WITH THE 2023 EDITION OF THE FLORIDA EXISTING

<u>TEMPORARY CONDITIONS:</u> THE STRUCTURAL INTEGRITY OF THE COMPLETED STRUCTURE DEPENDS ON INTERACTION OF VARIOUS CONNECTED COMPONENTS. PROVIDE ADEQUATE BRACING, SHORING, AND OTHER TEMPORARY SUPPORTS AS REQUIRED TO SAFELY COMPLETE THE WORK. THE STRUCTURE SHOWN ON THE DRAWINGS HAS BEEN DESIGNED FOR STABILITY UNDER FINAL CONFIGURATION ONLY.

EXISTING CONDITIONS: ALL EXISTING CONDITIONS, DIMENSIONS AND ELEVATIONS SHALL BE FIELD VERIFIED. THE CONTRACTOR SHALL NOTIFY THE ENGINEER OF ANY SIGNIFICANT DISCREPANCIES FROM CONDITIONS SHOWN ON THE DRAWINGS.

DESIGN CRITERIA: DESIGN WAS BASED ON STRENGTH AND DEFLECTION CRITERIA OF THE 2023 FLORIDA BUILDING CODE. THE FOLLOWING LOADS WERE USED FOR DESIGN, WITH LIVE LOADS REDUCED PER THE 2023 FBC.

SUPERIMPOSED DEAD LOADS

30 PSF 300 POUND CONCENTRATED

INCLUDES 5 PSF FOR SPRINKLER PIPING

SUPERIMPOSED LIVE LOADS:

STORAGE ABOVE CEILINGS 20 PSF

RAIN LOAD:

RAINFALL INTENSITY

WIND SPEED (ASCE 7-22)

160 MPH (124 MPH ALLOWABLE) RISK CATEGORY II

INTERNAL PRESSURE COEFF+/- 0.18

OPENINGS BELOW 60 FEET SHALL BE PROTECTED FROM WIND BORNE DEBRIS PER MISSILE LEVEL D OF ASTM E1996 TABLE 2 REQUIREMENTS.

FOUNDATIONS: FOUNDATION DESIGN IS BASED ON THE GEOTECHNICAL RECOMMENDATIONS IN THE REPORT PREPARED BY: PACSCON GEOENVIRONMENTAL, INC. PROJECT NUMBER: 2020-1357 DATED: MAY 21, 2020 WITH AN ALLOWABLE SOIL BEARING PRESSURE FOR SHALLOW FOUNDATIONS OF 2500 PSF. CONTRACTOR SHALL FOLLOW THE RECOMMENDATIONS SPECIFIED IN THE GEOTECHNICAL REPORT. THE GEOTECHNICAL ENGINEER SHALL EXAMINE THE EXCAVATION TO VERIFY SOIL CONDITIONS AND BEARING PRESSURE PRIOR TO PLACEMENT OF CONCRETE.

SUBMITTALS: SHOP DRAWINGS SHALL BE SUBMITTED TO THE ARCHITECT AND ENGINEER PRIOR TO FABRICATION AND CONSTRUCTION. SHOP DRAWINGS SHALL ADEQUATELY DEPICT THE STRUCTURAL ELEMENTS AND CONNECTIONS SHOWN ON THE CONTRACT DOCUMENTS.

SHOP DRAWINGS SHALL BE REVIEWED BY THE CONTRACTOR PRIOR TO SUBMITTAL TO THE ARCHITECT AND ENGINEER. DRAWINGS SUBMITTED WITHOUT REVIEW WILL BE RETURNED UNCHECKED. SHOP DRAWINGS WILL BE REVIEWED FOR GENERAL COMPLIANCE WITH THE DESIGN INTENT OF THE CONTRACT DOCUMENTS ONLY. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY COMPLIANCE WITH THE CONTRACT DOCUMENTS AS TO QUANTITY, LENGTH, ELEVATIONS, DIMENSIONS, ETC.

CONTRACTOR SHALL NOT BE RELIEVED FROM RESPONSIBILITY FOR ERRORS OR OMISSIONS IN SHOP DRAWINGS OR MIX DESIGNS BY THE ENGINEER'S REVIEW. IF A CONTRACTOR OR OWNER FAILS TO SUBMIT THE SHOP DRAWINGS, WOOLF ENGINEERING, WILL NOT BE RESPONSIBLE FOR THE STRUCTURAL CERTIFICATION AND DESIGN OF THE PROJECT.

REPRODUCTION OF STRUCTURAL DRAWINGS FOR USE AS SHOP DRAWINGS SHALL NOT BE PERMITTED. IF REPRODUCTION OF THESE CONTRACT DRAWINGS IN LIEU OF PREPARATION OF SHOP DRAWINGS OCCURS, IT SIGNIFIES ACCEPTANCE OF ALL INFORMATION SHOWN HEREON AS CORRECT, AND OBLIGATES CONTRACTOR, SUB-CONTRACTOR, ERECTOR, FABRICATOR, MATERIAL SUPPLIERS, ETC. TO ANY JOB EXPENSE, REAL OR IMPLIED, ARISING DUE TO ANY ERRORS THAT MAY OCCUR HEREON.

IF THE SHOP DRAWINGS DIFFER FROM OR ADD TO THE DESIGN OF THE STRUCTURAL DRAWINGS, THEY SHALL BEAR THE SEAL AND SIGNATURE OF A STRUCTURAL ENGINEER REGISTERED IN THE STATE OF THE PROJECT LOCATION. ANY CHANGES TO THE STRUCTURAL DRAWINGS SHALL BE SUBMITTED TO THE ARCHITECT AND ENGINEER AND ARE SUBJECT TO THE REVIEW AND ACCEPTANCE OF THE ENGINEER.

DESIGN DRAWINGS, SHOP DRAWINGS, AND CALCULATIONS FOR THE DESIGN AND FABRICATION OF ITEMS THAT ARE PREPARED BY A DELEGATED ENGINEER SHALL BE SIGNED AND SEALED BY AN ENGINEER REGISTERED IN THE STATE OF THE PROJECT LOCATION. CALCULATIONS SHALL BE INCLUDED.

CONTRACTOR SHALL ALLOW A MINIMUM PERIOD OF 10 WORKING DAYS FOR REVIEW OF STRUCTURAL SHOP DRAWINGS BY THE STRUCTURAL ENGINEER.

THE FOLLOWING SYSTEMS AND COMPONENTS AS A MINIMUM SHALL BE SUBMITTED FOR ENGINEER REVIEW:

CONCRETE MIX DESIGNS, CONCRETE AND MASONRY REINFORCING STEEL, FASTENERS, ANCHORS, BOLTS, AND EPOXY ANCHORS,

THE FOLLOWING SYSTEMS AND COMPONENTS AS A MINIMUM REQUIRE SHOP DRAWINGS PREPARED BY A **DELEGATED ENGINEER:**

PREMANUFACTURED WOOD TRUSSES,

PRE-ENGINEERED CANOPIES AND AWNINGS,

DEFERRED SUBMITTALS: IN ACCORDANCE WITH FBC 107.3.4.1, THE FOLLOWING SPECIALITY ITEMS FOR PORTIONS OF THE BUILDING WILL NOT BE SUBMITTED AT THE TIME OF BUILDING PERMIT APPLICATION BUT WILL BE DEFERRED UNTIL AFTER THE PERMIT HAS BEEN ISSUED.

PREMANUFACTURED WOOD TRUSSES INCLUDING TRUSS LAYOUT AND CALCULATIONS, PRE-ENGINEERED CANOPIES AND AWNINGS.

THESE ELEMENTS ARE PERFORMANCE-BASED DESIGN. THE CONTRACTOR SHALL CONTRACT FOR THE DESIGN AND CONSTRUCTION OF THESE ELEMENTS DURING THE CONSTRUCTION PHASE. THE SHOP DRAWINGS AND CALCULATIONS SHALL BE PREPARED AND SIGNED BY A STRUCTURAL ENGINEER REGISTERED IN THE STATE OF THE PROJECT LOCATION. THEY SHALL BE SUBMITTED FOR

REVIEW AND APPROVAL PRIOR TO FABRICATION. THE DEFERRED SUBMITTAL ITEMS SHALL NOT BE INSTALLED UNTIL THE DEFERRED SUBMITTAL DOCUMENTS HAVE BEEN APPROVED BY THE BUILDING OFFICIAL.

CONCRETE: REINFORCED CONCRETE CONSTRUCTION SHALL CONFORM TO THE FBC AND ACI 318 "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE". CONCRETE STRENGTHS SHALL BE VERIFIED BY STANDARD 28-DAY CYLINDER TESTS PER ASTM C39, AND SHALL BE AS FOLLOWS:

FOUNDATIONS/SLAB ON GRADE

4000 PSI ALL USES, U.N.O.

CEMENT SHALL CONFORM TO ASTM C150, TYPE 1. FLY ASH CONFORMING TO ASTM C618, TYPE F OR TYPE C, MAY BE USED TO REPLACE UP TO 20% OF THE CEMENT CONTENT, PROVIDED THAT THE MIX STRENGTH IS SUBSTANTIATED BY TEST DATA. COARSE AGGREGATE SHALL CONFORM TO ASTM C33 WITH A MAXIMUM SIZE OF 3/4". FINE AGGREGATE SHALL BE CLEAN, DURABLE, NATURAL SAND CONFORMING TO ASTM C33.

A WATER-REDUCING ADMIXTURE, IF USED, SHALL CONFORM TO ASTM C494 AND USED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS, SHALL BE INCORPORATED IN CONCRETE DESIGN MIXES. A HIGH-RANGE WATER-REDUCING ADMIXTURE CONFORMING TO ASTM C494, TYPE F OR G, MAY BE USED IN CONCRETE MIXES, PROVIDING THAT THE SLUMP DOES NOT EXCEED 8".

SLEEVES, OPENINGS, CONDUIT, AND OTHER EMBEDDED ITEMS NOT SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE APPROVED BY THE STRUCTURAL ENGINEER BEFORE POURING. NO SLEEVE, OPENING, OR INSERT MAY BE PLACED IN BEAMS, JOISTS, OR COLUMNS UNLESS APPROVED BY THE ENGINEER. CONDUITS EMBEDDED IN SLABS SHALL NOT BE LARGER IN OUTSIDE DIMENSION THAN ONE THIRD OF THE THICKNESS OF THE SLAB AND SHALL NOT BE SPACED CLOSER THAN THREE DIAMETERS ON CENTER.

PROVIDE 3/4" CHAMFERS ON ALL EXPOSED CONCRETE EDGES, UNLESS NOTED OTHERWISE. WHERE INDICATED OR REQUIRED, SLOPE CONCRETE SLABS TO DRAINS SHOWN ON PLUMBING AND/OR ARCHITECTURAL DRAWINGS.

ALL CONCRETE SHALL BE CURED IMMEDIATELY AFTER FINISHING OPERATIONS.

SHORING AND RESHORING: THIS PROJECT DOES NOT REQUIRE ENGINEERED SHORING AND RESHORING.

REINFORCING STEEL: REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60, FOR DEFORMED BAR AND ASTM A1064 FOR SMOOTH WELDED WIRE FABRIC (WWF), UNLESS OTHERWISE NOTED. REINFORCING STEEL TO BE WELDED SHALL CONFORM TO ASTM A706. REINFORCING STEEL SHALL BE SECURELY TIED IN PLACE WITH #16 ANNEALED IRON WIRE.

ALL DETAILING AND ACCESSORIES SHALL CONFORM TO ACI DETAILING MANUAL SP-66. PROVIDE CHAIRS, SPACERS, BOLSTERS, AND ITEMS IN CONTACT WITH FORMS WITH HOT-DIP GALVANIZED LEGS OR PLASTIC LEGS. ACCURATELY POSITION, SUPPORT, AND SECURE REINFORCEMENT AGAINST DISPLACEMENT BY FORMWORK CONSTRUCTION OR CONCRETE PLACEMENT OPERATIONS. "WET-STICKING" OF REINFORCING IS PROHIBITED.

REQUIRED CONCRETE COVER FOR REINFORCING STEEL (UNLESS NOTED OTHERWISE).

FOOTINGS 3" BOTTOM AND SIDES, 2" TOP

APPROVAL OF STRUCTURAL ENGINEER.

1-1/2" TO STIRRUPS

BEAMS

LAP SPLICE CONTINUOUS VERTICAL OR HORIZONTAL BARS IN CONCRETE MEMBERS IN ACCORDANCE WITH ACI 318, FOR CLASS "B" TENSION LAP SPLICES. DO NOT SPLICE CONTINUOUS TOP BARS IN BEAMS AT ENDS OF CLEAR SPANS. DO NOT SPLICE CONTINUOUS BOTTOM BARS IN BEAMS IN CLEAR SPANS BETWEEN SUPPORTS. SHOW ALL SPLICES ON SHOP DRAWINGS. SPLICE LOCATIONS AND METHODS SUBJECT TO

AT SLAB RE-ENTRANT CORNERS, PROVIDE (2) #5 X 4'-0" DIAGONAL BARS. AT SLAB AND WALL OPENINGS PROVIDE A MINIMUM OF (2) #5 BARS ALL FOUR SIDES AND DIAGONALLY; EXTEND THESE BARS A LAP DISTANCE OR A MINIMUM OF 24" PAST THE OPENING OR HOOK BARS IF DISCONTINUOUS.

DOWEL ALL WALLS AND COLUMNS TO FOOTINGS WITH BAR SIZE AND SPACING TO MATCH VERTICAL REINFORCING UNLESS OTHERWISE SHOWN.

SLABS ON GRADE: PREPARE SUBGRADE AS PER THE RECOMMENDATIONS OUTLINED IN THE GEOTECHNICAL REPORT. CHAIR WIRE FABRIC DURING CONCRETE PLACEMENT TO ENSURE PROPER POSITION IN SLAB. USE VAPOR BARRIER UNDER ALL ENCLOSED INTERIOR SPACES, PER ARCHITECTURAL DRAWINGS.

PLACE CRACK CONTROL JOINTS AS SHOWN ON PLAN OR AT 12 FEET MAXIMUM FOR 4" SLAB, OR 15 FEET MAXIMUM FOR 6" SLAB. JOINT SPACING SHALL NOT EXCEED A 1.5 TO 1 WIDTH TO LENGTH RATIO. CONTRACTOR SHALL SUBMIT A CONTROL JOINT LAYOUT FOR ENGINEER'S AND ARCHITECT'S REVIEW PRIOR TO CONCRETE PLACEMENT. LOCATE CONTROL JOINTS AT COLUMN LINES AND RE-ENTRANT CORNERS TYPICAL. PROVIDE (1) #5 X 4'-0" DIAGONAL BARS AT SLAB RE-ENTRANT CORNERS.

FOR 4" THICK SLABS ON GRADE, PROVIDE 6X6 W1.4XW1.4 WELDED WIRE FABRIC OR 1.5 POUNDS PER CUBIC YARD OF MICRO SYNTHETIC FIBERS (FRC MONO-150, DUOMIX M20, OR EQUAL), UNLESS NOTED OTHERWISE FOR 6" THICK SLABS ON GRADE, PROVIDE 6X6 W2.9XW2.9 WELDED WIRE FABRIC PLACED 2" BELOW TOP OF SLAB OR 3 POUNDS PER CUBIC YARD OF MACRO SYNTHETIC FIBERS (FORTA FERRO, SYNMIX T57, OR EQUAL), UNLESS NOTED OTHERWISE.

POST-INSTALLED ANCHORS INTO CONCRETE AND MASONRY: POST-INSTALLED ANCHORS SHALL ONLY BE USED WHERE SPECIFIED ON THE CONSTRUCTION DOCUMENTS. THE CONTRACTOR SHALL OBTAIN APPROVAL FROM THE ENGINEER-OF-RECORD (EOR) PRIOR TO INSTALLING POST INSTALLED ANCHORS IN PLACE OF MISSING OR MISPLACED CAST-IN-PLACE ANCHORS.

IDENTIFY POSITION OF REINFORCING STEEL AND OTHER EMBEDDED ITEMS PRIOR TO DRILLING HOLES FOR ANCHORS. CARE SHALL BE TAKEN IN PLACING POST-INSTALLED ANCHORS TO AVOID CONFLICTS WITH EXISTING REBAR AND/OR OTHER EMBEDDED ITEMS. HOLES SHALL BE DRILLED AND CLEANED IN ACCORDANCE WITH THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS. NOTIFY THE ENGINEER-OF-RECORD IF REINFORCING STEEL OR OTHER EMBEDDED ITEMS ARE ENCOUNTERED DURING DRILLING.

SUBSTITUTION REQUESTS FOR PRODUCTS OTHER THAN THOSE SPECIFIED BELOW SHALL BE SUBMITTED BY THE CONTRACTOR TO THE ENGINEER-OF-RECORD ALONG WITH CALCULATIONS THAT ARE PREPARED & SEALED BY A REGISTERED PROFESSIONAL ENGINEER. THE CALCULATIONS SHALL DEMONSTRATE THAT THE SUBSTITUTED PRODUCT IS CAPABLE OF ACHIEVING EQUIVALENT PERFORMANCE (MINIMUM) VALUES OF THE SPECIFIED PRODUCT USING THE APPROPRIATE DESIGN PROCEDURE AND/OR STANDARD(S).

POST-INSTALLED ANCHORS SHALL HAVE THE ICC-ES EVALUATION REPORT INDICATING CONFORMANCE WITH CURRENT APPLICABLE ICC ES ACCEPTANCE CRITERIA.

PROVIDE SPECIAL INSPECTIONS AS REQUIRED BY THE ANCHOR'S EVALUATION REPORT (ICC-ES ESR OR IAPMO-UES ER). CONTACT MANUFACTURER'S REPRESENTATIVE FOR THE INITIAL TRAINING AND INSTALLATION OF ANCHORS AND FOR PRODUCT RELATED QUESTIONS AND AVAILABILITY.

UNLESS OTHERWISE NOTED, PROVIDE CARBON STEEL ANCHORS, ZINC PLATED ACCORDING TO ASTM B633, THREADED STUDS ACCORDING TO ASTM A36, OR HOT-DIPPED GALVANIZED ACCORDING TO ASTM A153. PERMANENTLY EXPOSED ANCHORS SHALL BE STAINLESS STEEL. NUTS AND WASHERS SHALL CONFORM TO SAME SPECIFICATION AS THE SUPPLIED ANCHOR RODS. USE 316 STAINLESS STEEL FOR MECHANICAL EQUIPMENT AND FOR ALUMINUM AND FIBERGLASS STRUCTURES AND ASSEMBLIES.

INSTALLATION SHALL BE IN CONFORMANCE WITH MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS. EMBEDMENT SHALL BE AS INDICATED ON THE STRUCTURAL DRAWINGS.

MINIMUM EMBEDMENT FOR MECHANICAL ANCHORS SHALL BE AS FOLLOWS, OR AS NOTED ON THE DRAWINGS:

1. 1/2" DIAMETER - 3 3/4" EMBEDMENT

2. 5/8" DIAMETER - 5" EMBEDMENT 3. 3/4" DIAMETER - 5 3/4" EMBEDMENT

FOR ANCHORING INTO CRACKED AND UNCRACKED CONCRETE

A.MECHANICAL ANCHORS: SHALL HAVE BEEN TESTED IN ACCORDANCE WITH ACI 355.2 AND/OR ICC-ES AC193 FOR CRACKED AND UNCRACKED CONCRETE, PRE-APPROVED PRODUCTS INCLUDE:

1. HILTI KWIK HUS EZ SCREW ANCHORS PER ICC-ES ESR 3027 2. DEWALT SCREW-BOLT+ SCREW ANCHOR PER ICC ES ESR 3889 3. SIMPSON STRONG-TIE TITEN HD SCREW ANCHOR PER ICC ES ESR 2713 AND IAPMO UES ER 493 4. SIMPSON STRONG-TIE TITEN TURBO PER IAPMO UES ER 712

B. ADHESIVE ANCHORS: SHALL HAVE BEEN TESTED IN ACCORDANCE WITH ACI 355.4 AND ICC-ES AC308 FOR CRACKED AND UNCRACKED CONCRETE. ADHESIVE ANCHORS SHALL BE INSTALLED IN CONCRETE HAVING A MINIMUM AGE OF 21 DAYS. HOLES SHALL BE DRY AT THE TIME OF INSTALLATION. ACI 355.4 TEMPERATURE CATEGORY 'B' ASSUMED IN DESIGN. PRIOR TO INSTALLATION OF ADHESIVE ANCHORS IN HORIZONTAL OR UPWARDLY INCLINED ORIENTATIONS RESISTING SUSTAINED TENSION LOADS, INSTALLERS ARE REQUIRED TO BE CERTIFIED IN ACCORDANCE WITH THE ACI/CRSI ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM AND MUST BE CONTINUOUSLY INSPECTED. PRE-APPROVED PRODUCTS INCLUDE:

ACCECTABLE ADHESIVE IN FOUNDATIONS, SLAB ON GRADE, COLUMNS AND WALLS ARE:

- 1. HILTI HY200 PER ICC ES ESR 4868
- 2. DEWALT AC200+ PER ICC ES ESR 4027
- 3. SIMPSON STRONG-TIE SET-3G PER ICC ES ESR 4057

ACCEPTABLE ADHESIVE IN BEAMS AND ELEVATED SLABS ARE:

- 1. HILTI HIT RE500 V3 PER ICC ELC 3814
- 2. DEWALT PURE110+ PER ICC ES ESR 3298
- 3. SIMPSON STRONG-TIE SET-3G PER ICC-ES ESR 4057

FOR ANCHORING INTO GROUT-FILLED CONCRETE MASONRY UNITS

- A.MECHANICAL ANCHORS SHALL HAVE BEEN TESTED IN ACCORDANCE WITH ICC ES AC106 (SCREW ANCHORS). PRE-APPROVED PRODUCTS INCLUDE:
- 1. HILTI KWIK HUS EZ SCREW ANCHORS PER ICC ES ESR 3056
- 2. DEWALT SCREW-BOLT+ SCREW ANCHOR PER ICC ES ESR

3. SIMPSON STRONG-TIE TITEN HD SCREW ANCHOR PER ICC

4. SIMPSON STRONG-TIE TITEN TURBO PER IAPMO UES ER 716

B. ADHESIVE ANCHORS SHALL HAVE BEEN TESTED IN ACCORDANCE

WITH ICC ES AC58. PRE-APPROVED PRODUCTS INCLUDE:

- 1. HILTI HY200 PER ICC ES ESR 4878
- 2. DEWALT AC100+ GOLD PER ICC ES ESR 3200
- 3. SIMPSON STRONG-TIE SET-3G PER ICC ES ESR 4057

MASONRY WALLS: MASONRY UNITS SHALL MEET ASTM C90, TYPE 2. ASSEMBLIES SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF f'm= 2,000 PSI. MORTAR SHALL BE TYPE "M" OR "S" AND MEET ASTM C270. GROUT SHALL MEET ASTM C476. GROUT STRENGTHS SHALL BE VERIFIED BY STANDARD 28-DAY TESTS PER ASTM C1019. GROUT SHALL CONSIST OF A MIXTURE OF CEMENTITIOUS MATERIALS AND AGGREGATE TO WHICH SUFFICIENT WATER HAS BEEN ADDED TO CAUSE THE MIXTURE TO FLOW WITHOUT SEGREGATION OF THE CONSTITUENTS. ALL CELLS CONTAINING VERTICAL BARS, BOND BEAMS, AND ALL CELLS BELOW GRADE SHALL BE FILLED WITH GROUT. MAXIMUM HEIGHT OF GROUT POUR ALLOWED IS 4'-0" UNLESS CLEAN OUT OPENING IS PROVIDED AT BOTTOM OF CELLS TO BE FILLED. LOCATE CLEAN-OUT OPENINGS IN AREAS NOT EXPOSED TO VIEW.

UNLESS NOTED OTHERWISE EIGHT INCH MASONRY WALLS SHALL BE PARTIALLY REINFORCED MASONRY WALL CONSTRUCTION WITH #5 AT 48 INCH O.C. IN GROUT FILLED CELLS. ADD (2) #5 REINFORCING BAR UNDER EACH TRUSS GIRDER, TYPICAL UNLESS NOTED OTHERWISE.

PROVIDE REINFORCING BARS AT CORNERS, INTERSECTIONS, AND EACH SIDE OF OPENINGS. PROVIDE (2) REINFORCING BARS EACH SIDE OF OPENINGS OVER 4 FEET WIDE, AND AS SHOWN ON THE PLANS. PROVIDE HOOKED DOWELS INTO FOOTINGS AND STRUCTURE ABOVE AND/OR BELOW TO PROVIDE CONTINUITY. PROVIDE 9 GAGE GALVANIZED HORIZONTAL JOINT REINFORCING (DUR-O_WAL OR ENGINEER-APPROVED EQUAL) AT 16" O.C. REINFORCING LAPS TO BE 48 BAR DIAMETERS.

DO NOT PLACE CONDUITS, PIPES, ETC., IN CELLS WITH VERTICAL REINFORCING. DO NOT RUN CONDUITS PIPES, ETC., HORIZONTALLY IN CMU WALLS PARALLEL TO LENGTH OF WALL. WHERE MASONRY WALLS ABUT CONCRETE COLUMNS TO BE PLACED PRIOR TO ERECTION OF MASONRY WALLS, PROVIDE DOVETAIL SLOTS BETWEEN COLUMN AND WALLS AND GROUT THE CMU CELL CONTAINING THE DOVETAIL ANCHORS. OTHERWISE, EXTEND CMU HORIZONTAL JOINT REINFORCING THROUGH CONCRETE COLUMN.

CONTROL JOINTS SHALL BE PROVIDED IN ALL CONCRETE MASONRY CONSTRUCTION AT A SPACING NOT TO EXCEED THREE TIMES WALL HEIGHT OR 30'-0" MAXIMUM. COORDINATE LOCATIONS WITH THE ARCHITECTURAL DRAWINGS. HORIZONTAL WALL REINFORCING SHALL BE STOPPED EACH SIDE OF CONTROL JOINTS. SEE ARCHITECTURAL DRAWINGS FOR SEALANT REQUIREMENTS AT CONTROL JOINTS.

USE METAL LATH OR WIRE SCREEN FOR CAVITY CAPS. SHEET METAL, FELT, BUILDING PAPER, OR LIKE MATERIALS ARE PROHIBITED.

TIE BEAMS: TIE BEAMS SHALL BE CONCRETE, POURED AFTER THE BLOCK WALLS BELOW ARE IN PLACE. REINFORCING SHALL BE CONTINUOUS THROUGH TIE BEAMS WITH MINIMUM LAP SPLICES OF 48 BAR DIAMETERS AND BENT BARS AT CORNERS. USE METAL LATH, MORTAR, OR SPECIAL UNITS TO CONFINE CONCRETE TO AREA REQUIRED, IN ACCORDANCE WITH ACI 530.1, SECTION 3.5 B. SOLID METAL OR FELT CAVITY CAPS ARE PROHIBITED.

PRECAST CONCRETE LINTELS: UNLESS INDICATED OTHERWISE, ALL LINTELS TO BE "U" TYPE PRECAST CONCRETE UNITS EQUAL TO UNITS MANUFACTURED BY CAST_CRETE CORP. AND PRESTRESSED (AND ADDITIONALLY REINFORCED AS REQUIRED) IN ACCORDANCE WITH CAST CRETE CORP. "DESIGN MANUAL", LATEST EDITION, FOR THE SPAN AND LOADING CONDITION RELATIVE TO LINTEL LOCATION.

LINTEL SIZE IF NOT SHOWN ON THE PLANS SHALL BE 8F16-1B/1T FOR OPENINGS LESS THAN 6 FEET AND 8F24-1B/1T FOR OPENINGS 6 FEET TO 10 FEET. PROVIDE 8" MINIMUM BEARING FOR LINTELS UNLESS NOTED OTHERWISE.

SAWN LUMBER: SAWN LUMBER SHALL BE SOUTHERN PINE #2 WITH THE ALLOWABLE FIBER STRESSES PER THE AWC NATIONAL DESIGN SPECIFICATION.

ALL LUMBER EXPOSED TO WEATHER SHALL BE PROTECTED OR PRESSURE TREATED. ALL LUMBER IN CONTACT WITH CONCRETE OR MASONRY SHALL BE PROTECTED OR PRESSURE TREATED.

ALL FRAMING NAILS SHALL BE COMMON NAILS AND SHALL BE OF THE SIZE AND NUMBER INDICATED ON THE DRAWINGS. NAILING NOT SHOWN SHALL BE AS INDICATED IN TABLE 2304.10.1 OF THE FBC. BOLTS AND LAG SCREWS SHALL CONFORM TO ANSI/ASME STANDARD B18.1. ALL BOLTS AND LAG SCREWS SHALL BE INSTALLED WITH STANDARD CUT WASHERS.

WOOD FRAMING CONNECTORS: FRAMING ACCESSORIES AND STRUCTURAL FASTENERS SHALL BE MANUFACTURED BY SIMPSON COMPANY (OR APPROVED EQUAL) AND OF THE SIZE AND TYPE SHOWN ON THE DRAWINGS. HANGERS NOT SHOWN SHALL BE SIMPSON HU OF SIZE RECOMMENDED FOR MEMBER. ALL CONNECTORS SHALL BE GALVANIZED. UNLESS SHOWN OTHERWISE, INSTALL MAXIMUM SIZE AND NUMBER OF FASTENERS SHOWN IN LATEST SIMPSON CATALOG.

HEADERS AND LEDGERS: HEADERS AND LEDGERS LOADED FROM TOP SHALL BE CONNECTED TOGETHER WITH 2 ROWS SIMPSON SDW SCREWS AT 16" OC THROUGH ALL PLYS WITH 1 3/8" MINUMUM EMBEDMENT. HEADERS AND LEDGERS LOADED BY FACE-MOUNTED BUCKETS SHALL BE CONNECTED TOGETHER AS FOLLOWS: 2X6 AND 2X8 CONNECTED TOGETHER WITH 2 ROWS SIMPSON SDW SCREWS AT 12" OC, 2X10 AND 2X12 CONNECTED TOGETHER WITH 3 ROWS SIMPSON SDW SCREWS AT 12" OC.

SDW SCREWS SHALL BE INSTALLED WITH 1 1/2" EDGE DISTANCE, 6" END DISTANCE, AND 4" MINIMUM CENTER TO CENTER.

PREFABRICATED WOOD TRUSSES: DESIGN AND MANUFACTURE IN ACCORDANCE WITH TPI "DESIGN

SPECIFICATIONS FOR METAL PLATE CONNECTED WOOD TRUSSES". TRUSS DESIGNER TO DETERMINE AND ESTABLISH HEIGHT, LENGTH, LOCATION, SPACING, REQUIRED BEARING WIDTH, REACTIONS, AND REQUIRED PERMANENT BRACING FOR EACH TRUSS. COORDINATE WITH ARCHITECTURAL AND MECHANICAL DRAWINGS FOR ADDITIONAL ITEMS INCLUDING AIR HANDLER

LOCATIONS, MECHANICAL ROOMS AND DUCT SPACE AND ROUTING. TRUSS BOTTOM CHORD IS NOT BRACED BY CEILING. DESIGN BOTTOM CHORD TO BE UNBRACED OR PROVIDE BRACING.

PRIOR TO FABRICATION, SUBMIT COMPLETE SHOP DRAWINGS AND CALCULATIONS, SIGNED AND SEALED BY A LICENSED STRUCTURAL ENGINEER, SUBSTANTIATING ALL STRENGTH AND SERVICEABILITY CRITERIA. DESIGN LOADS SHALL BE CLEARLY INDICATED ON SHOP DRAWINGS AND LOADS IMPOSED UPON THE STRUCTURAL SYSTEM.

TRUSSES SHALL BE DESIGNED FOR A 300 LB POINT LOAD AT ANY LOCATION AND ALL SHEAR TRUSSES SHALL BE DESIGNED FOR A 1,550 LB HORIZONTAL POINT LOAD, UNLESS NOTED OTHERWISE. TRUSS LOADING SHALL BE AS FOLLOWS, IN ADDITION TO LOADS SHOWN ON THE DRAWINGS:

ROOF TRUSS LOADING:

TOP CHORD LIVE LOAD 20 PSF OR WIND UPLIFT SHOWN ON DRAWINGS

TOP CHORD DEAD LOAD 25 PSF BOTTOM CHORD DEAD LOAD

BOTTOM CHORD LIVE LOAD WIND UPLIFT WHERE EXPOSED (I.E., LANAIS, ENTRIES, PORCHES,

TRUSS SHEAR PANELS/BLOCKING

WHERE THE DISTANCE FROM THE TOP OF THE BRACED WALL TO THE TOP OF THE ROOF TRUSS ABOVE IS 9 1/4" OR LESS, BLOCKING BETWEEN ROOF TRUSSES IS NOT REQUIRED, UNLESS NOTED OTHERWISE.

BALCONIES, ETC.)

WHERE THE DISTANCE FROM THE TOP OF THE BRACED WALL TO THE TOP OF THE ROOF TRUSS IS GREATER THAN 9 1/4" AND LESS THAN 15 1/4". SOLID BLOCKING BETWEEN ROOF TRUSSES IS REQUIRED. PROVIDE THE MAXIMUM HEIGHT SOLID BLOCKING ALLOWED. MAINTAIN A GAP ABOVE THE BLOCKING FOR VENTILATION (2" MAXIMUM), UNLESS NOTED OTHERWISE.

WHERE THE DISTANCE FROM THE TOP OF THE BRACED WALL TO THE TOP OF THE ROOF TRUSS IS GREATER THAN 15 1/4" TRUSS SHEAR PANELS BETWEEN ROOF TRUSSES IS REQUIRED. TRUSS SHEAR PANELS SHALL BE FULL HEIGHT AND DESIGNED FOR A MINIMUM HORIZONTAL ALLOWBALE LOAD OF 1,550 LBS, UNLESS

<u>PLYWOOD:</u> PLYWOOD PANELS SHALL CONFORM TO THE REQUIREMENTS OF "U.S. PRODUCT STANDARD PS-1 FOR CONSTRUCTION AND INDUSTRIAL PLYWOOD" OR APA PRP-108 PERFORMANCE STANDARDS. UNLESS OTHERWISE NOTED, PANELS SHALL BE APA RATED SHEATHING, EXPOSURE 1, OF THE THICKNESS AND SPAN RATING SHOWN ON THE DRAWINGS.

PLYWOOD INSTALLATION SHALL BE IN CONFORMANCE WITH APA RECOMMENDATIONS. ALLOW 1/8" SPACING AT PANEL EDGES, UNLESS OTHERWISE RECOMMENDED BY THE PANEL MANUFACTURER. ALL SHEATHING SHALL BE INSTALLED WITH FACE GRAIN PERPENDICULAR TO SUPPORTS, EXCEPT AS

INDICATED ON THE DRAWINGS. STAGGER ENDS OF ADJACENT PANELS 4'-0".

ROOF SHEATHING SHALL BE 5/8" PLYWOOD, BLOCKED, TONGUE-AND-GROOVE, OR HAVE EDGES SUPPORTED BY PLYCLIPS. ATTACH PLYWOOD PANELS TO SUPPORTING MEMBERS WITH 8d RINGSHANK NAILS SPACED 4" ON CENTER ALONG THE PANEL EDGES AND AT 6" ON CENTER ALONG INTERMEDIATE SUPPORTS, UNLESS NOTED OTHERWISE.

<u>SOFFITS:</u> SOFFITS SHALL BE CAPABLE OF RESISTING THE DESIGN PRESSURES FOR WALLS SPECIFIED IN THE COMPONENT AND CLADDING CHART. SOFFITS SHALL BE CONSTRUCTED WITH 2x4 AT 24" OC WITH 1/2" PLYWOOD WITH 8d @ 4" OC EDGES AND 6" OC FIELD OR PER FLORIDA PRODUCT APPROVAL.

	ALLOWABLE COMPONENT & CLADDING WIND PRESSURES (PSF)						
	ZONE			TRIBUTARY AREA			
	ZONE		10 SF	50 SF	100 SF		
	INTERIOR	1	26 / -47	18 / -34	14 / -29		
ROOF	RIDGE/EDGE	2e & 2r	26 / -65	18 / -44	14 / -44		
	CORNER	3	26 / -65	18 / -44	14 / -47		
WALL	INTERIOR	4	35 / -38	35 / -38	30 / -33		
	CORNER	5	35 / -47	35 / -47	30 / -36		

1. PLUS AND MINUS SIGNS SIGNIFY PRESSURES ACTING TOWARD AND AWAY FROM THE BUILDING

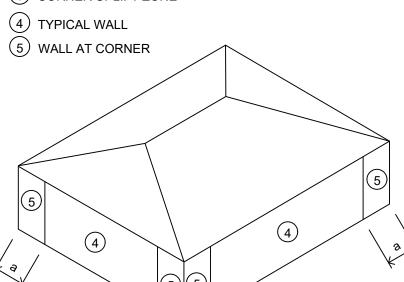
2. EDGE DISTANCE 'a' = 7.4 FT

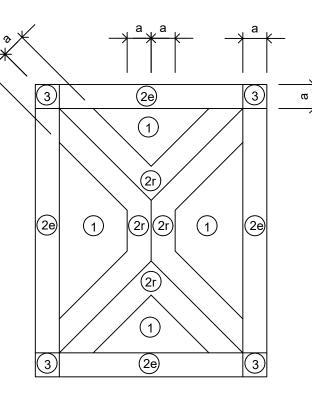
3. ALL WINDOWS AND DOORS TO BE DESIGNED FOR +35 PSF / -47 PSF ALLOWABLE WIND PRESSURE.

4. ALL GARAGE DOOR OR OVERHEAD DOORS TO BE DESIGNED FOR +35 PSF / -38 PSF ALLOWABLE WIND

PORT CHARLOTTE, FL" PH: (941) 764-0200 info@woolfeng.com

(1) GENERAL UPLIFT ZONE (2) END UPLIFT ZONE (3) CORNER UPLIFT ZONE





This document has been digitally signed using a SHA256 authentication code. Printed copies of this document are not considered signed and sealed

WOOLF ENGINEERING BEFORE COMMENCEMENT OF THE PROJECT. CHANGES MADE TO THESE PLANS BY THE G.C. OR OTHERS ARE THE SOLE RESPONSIBILITY OF THE GENERAL CONTRACTOR. THE CONCEPTUAL DESIGN AND LAYOUT FOR THIS STRUCTURE HAS BEEN PROVIDED TO WOOLF ENGINEERING BY THE OWNER OR CLIENT ND ANY SIMILARITIES TO ANOTHER PLAN OR DESIGN IS THE TOTAL RESPONSIBILITY OF THE OWNER OR CLIENT. AFFIXED SEAL WITH ENGINEER'S SIGNATURE CERTIFIES THAT THE STRUCTURAL

GENERAL CONTRACTOR TO VERIFY ALL DIMENSIONS, DETAILS, AND SPECIFICATIONS

SHOWN. OMISSIONS AND/OR ERRORS SHALL BE BROUGHT TO THE ATTENTION OF

COMPONENTS OF THIS PLAN ARE IN COMPLIANCE TO THE FLORIDA BUILDING CODES 8TH EDITION (2023). RESISTANCE TO WIND PRESSURE IS IN COMPLIANCE WITH SECTION 1609. SIGNED/SEALED PLANS ARE VALID FOR ONE YEAR ONLY FROM DATE 20020 VETERANS BLVD, #20



Port Charlotte, Florida 33952 Ph. (941) 639-2450 Fax (941) 639-2438 3820 Colonial Blvd. #10 Fort Myers, Florida 3396



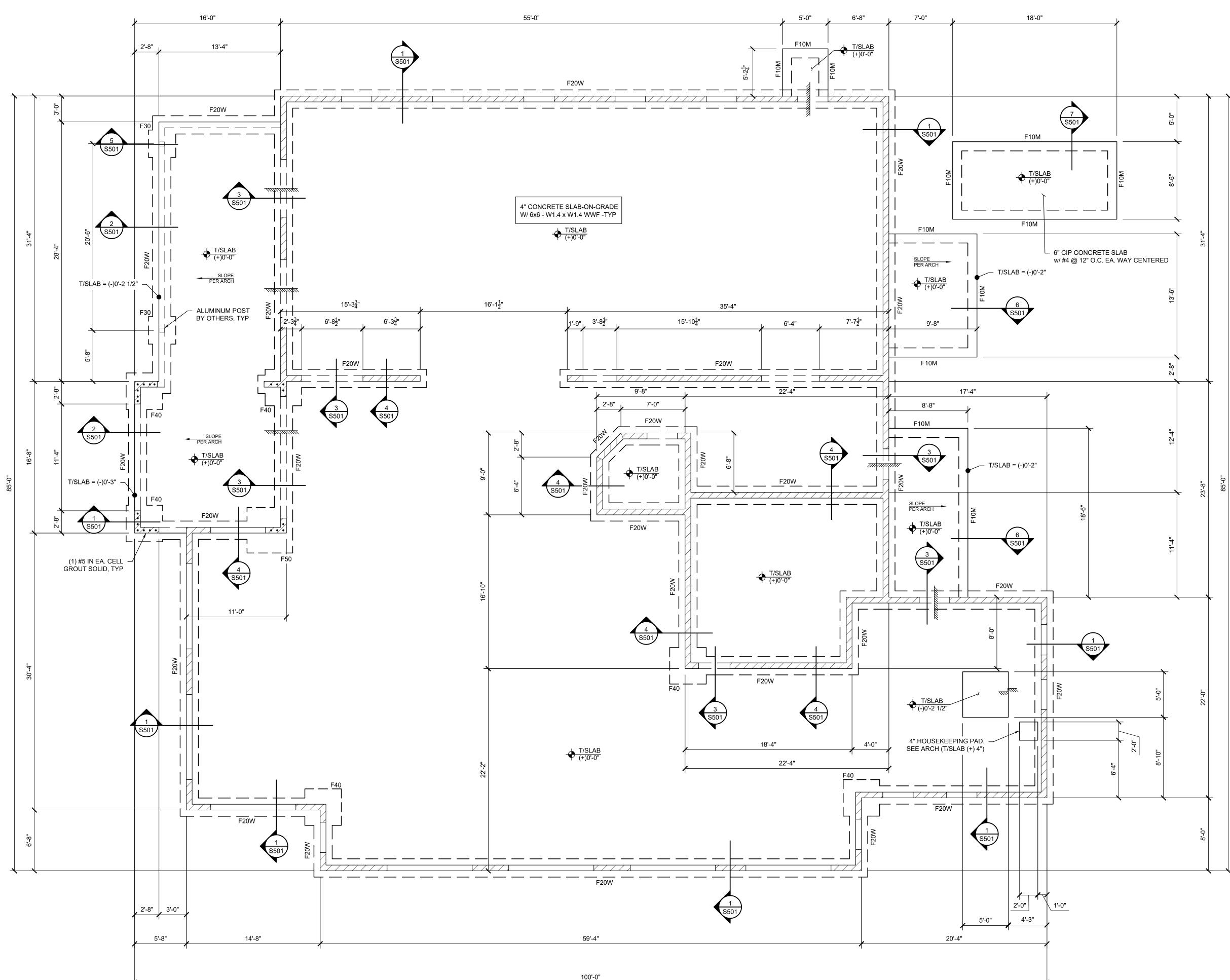
COREY M. SOLICH - PE 96895 REGISTRY # 35292

ROJECT NO. 2024-J-104

PROJECT NO. 2024-J-104

08 - 08 - 2024

COREY M. SOLICH - PE 96895 REGISTRY # 35292





CONCRETE FOOTING SCHEDULE

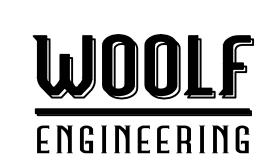
MARK	SIZE	DEPTH	REINFORCING
F10M	1'-0" x CONT.	1'-0"	(2) #5 CONTINUOUS
F20W	2'-0" x CONT.	1'-0"	(3) #5 CONTINUOUS
F30	3'-0" x 3'-0"	1'-0"	(4) #5 EACH WAY BOTTOM
F40	4'-0" x 4'-0"	1'-0"	(6) #5 EACH WAY BOTTOM
F50	5'-0" x 5'-0"	1'-0"	(7) #5 EACH WAY BOTTOM
	_		_

NOTES:

1. 'M' DENOTES A MONOLITHIC FOOTING.

FOUNDATION PLAN NOTES:

- 1. SEE SHEET S001 FOR GENERAL STRUCTURAL NOTES.
- 2. DO NOT SCALE DRAWINGS. VERIFY / COORDINATE ALL DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL DRAWINGS BEFORE COMMENCING CONSTRUCTION. NOTIFY STRUCTURAL ENGINEER AND ARCHITECT OF ANY DISCREPANCIES BEFORE PROCEEDING WITH WORK.
- CONTRACTOR TO VERIFY ALL EXISTING CONDITIONS, DIMENSIONS AND ELEVATIONS PRIOR TO CONSTRUCTION AND NOTIFY STRUCTURAL ENGINEER AND ARCHITECT OF ANY DISCREPANCIES BEFORE PROCEEDING WITH WORK.
- 4. SEE ARCHITECTURAL DRAWINGS FOR SLOPES, DROPS AND DRAIN LOCATION IN FLOOR SLABS.
- 5. VERIFY / COORDINATE EDGE OF SLAB DETAILS AT EXTERIOR DOORS WITH ARCHITECTURAL DRAWINGS.
- 6. SEE GEOTECHNICAL RECOMMENDATIONS FOR SUBGRADE COMPACTION, VAPOR BARRIER AND DRAINAGE REQUIREMENTS.
- VERIFY / COORDINATE LOCATION OF UNDERGROUND PIPING WITH
- VERIFY / COORDINATE SILL HEIGHTS AND DETAILS OF WALL OPENINGS WITH ARCHITECTURAL DRAWINGS.
- 9. FX INDICATES FOOTING TYPE, SEE SCHEDULE THIS SHEET.
- 10. X'-X" INDICATES TOP OF FOOTING ELEVATION, -1'-4" UNLESS NOTED OTHERWISE.
- 11. INDICATES STEP IN SLAB ELEVATION.
- 12. INDICATES 8" CMU WALL W/ #5 VERTICALS IN GROUTED CELLS AT 48" OC MAX AND AT CORNERS, INTERSECTIONS AND EACH SIDE OF OPENINGS.



PH: (941) 764-0200 info@woolfeng.com

This document has been digitally signed using a SHA256 authentication code. Printed copies of this document are not considered signed and sealed GENERAL CONTRACTOR TO VERIFY ALL DIMENSIONS, DETAILS, AND SPECIFICATIONS SHOWN. OMISSIONS AND/OR ERRORS SHALL BE BROUGHT TO THE ATTENTION OF WOOLF ENGINEERING BEFORE COMMENCEMENT OF THE PROJECT. CHANGES MADE TO THESE PLANS BY THE G.C. OR OTHERS ARE THE SOLE RESPONSIBILITY OF THE GENERAL CONTRACTOR. THE CONCEPTUAL DESIGN AND LAYOUT FOR THIS STRUCTURE HAS BEEN PROVIDED TO WOOLF ENGINEERING BY THE OWNER OR CLIENT AND ANY SIMILARITIES TO ANOTHER PLAN OR DESIGN IS THE TOTAL RESPONSIBILITY OF THE OWNER OR CLIENT.

OF THE OWNER OR CLIENT. AFFIXED SEAL WITH ENGINEER'S SIGNATURE CERTIFIES THAT THE STRUCTURAL COMPONENTS OF THIS PLAN ARE IN COMPLIANCE TO THE FLORIDA BUILDING CODES 8TH EDITION (2023). RESISTANCE TO WIND PRESSURE IS IN COMPLIANCE WITH SECTION 1609. SIGNED/SEALED PLANS ARE VALID FOR ONE YEAR ONLY FROM DATE 20020 VETERANS BLVD, #20 SECTION SIGNED. PORT CHARLOTTE, FL"

ans are not to be to obtaining the steemit

INS. THESE PLANS

INDICATES PRE-ENGINEERED WOOD TRUSSES @ 24" OC.
INDICATES PRE-ENGINEERED WOOD TRUSS GIRDER.

2'-0" (2) #5 (2) #5 (2) #5

2'-0" (2) #6 (2) #5 (2) #6

1. PROVIDE PRECAST LINTEL BY CASTCRETE OR APPROVED EQUAL, TYPICAL

2. PROVIDE #3 STIRRUPS @ 48" OC IN CONCRETE BEAMS, TYPICAL UNLESS

2. DO NOT SCALE DRAWINGS. VERIFY / COORDINATE ALL DIMENSIONS AND

3. CONTRACTOR TO VERIFY ALL EXISTING CONDITIONS, DIMENSIONS AND

ENGINEER AND ARCHITECT OF ANY DISCREPANCIES BEFORE

NAILS AT 6" OC AND EDGES AT 4" OC.

ELEVATIONS PRIOR TO CONSTRUCTION AND NOTIFY STRUCTURAL

INDICATES CONCRETE BEAM TYPE, SEE SCHEDULE THIS

INDICATES SPAN DIRECTION OF 5/8" PLYWOOD SHEATHING. ATTACH TO INTERMEDIATE SUPPORTS W/ 8d RINGSHANK

SPAN DIRECTION OF 8" CIP CONCRETE DECK w/ #5 @ 6" OC

ANY DISCREPANCIES BEFORE PROCEEDING WITH WORK.

ELEVATIONS WITH ARCHITECTURAL DRAWINGS BEFORE COMMENCING CONSTRUCTION. NOTIFY STRUCTURAL ENGINEER AND ARCHITECT OF

UNLESS NOTED OTHERWISE, BELOW BEAMS AND ABOVE MASONRY

1. SEE SHEET S001 FOR GENERAL STRUCTURAL NOTES.

4. TX'-X" INDICATES TOP OF BEAM ELEVATION.

9. TG INDICATES PRE-ENGINEERED WOOD TRUSS GIRI

10. _____ INDICATES CONCRETE BEAM OR WALL BELOW.

11. INDICATES 8" CMU WALL W/ #5 VERTICALS IN GROUTED CELLS AT 48" OC MAX AND AT CORNERS, INTERSECTIONS AND EACH SIDE OF OPENINGS.

12. PROVIDE CONTINUOUS UNINTERRUPTED ROOF SHEATHING UNDER OVER-BUILDS.

3. INDICATES STEP IN WALL ELEVATION.

This document has been digitally signed using a SHA256 authentication code. Printed copies of this document are not considered signed and sealed CONTRACTOR TO VERIFY ALL DIMENSIONS, DETAILS, AND SPECIFICATIONS

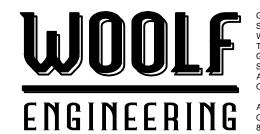
STIRRUP SPACING

#3 @ 48" O.C.

#3 @ 6" O.C.

#3 @ 6" O.C.

#3 @ 6" O.C.



PORT CHARLOTTE, FL" PH: (941) 764-0200 info@woolfeng.com

CONCRETE BEAM SCHEDULE

OPENINGS AS PER LINTEL SCHEDULE.

NOTED OTHERWISE.

ROOF FRAMING PLAN NOTES:

PROCEEDING WITH WORK.

GENERAL CONTRACTOR TO VERIFY ALL DIMENSIONS, DETAILS, AND SPECIFICATIONS SHOWN. OMISSIONS AND/OR ERRORS SHALL BE BROUGHT TO THE ATTENTION OF WOOLF ENGINEERING BEFORE COMMENCEMENT OF THE PROJECT. CHANGES MADE TO THESE PLANS BY THE G.C. OR OTHERS ARE THE SOLE RESPONSIBILITY OF THE GENERAL CONTRACTOR. THE CONCEPTUAL DESIGN AND LAYOUT FOR THIS STRUCTURE HAS BEEN PROVIDED TO WOOLF ENGINEERING BY THE OWNER OR CLIENT AND ANY SIMILARITIES TO ANOTHER PLAN OR DESIGN IS THE TOTAL RESPONSIBILITY OF THE OWNER OR CLIENT.

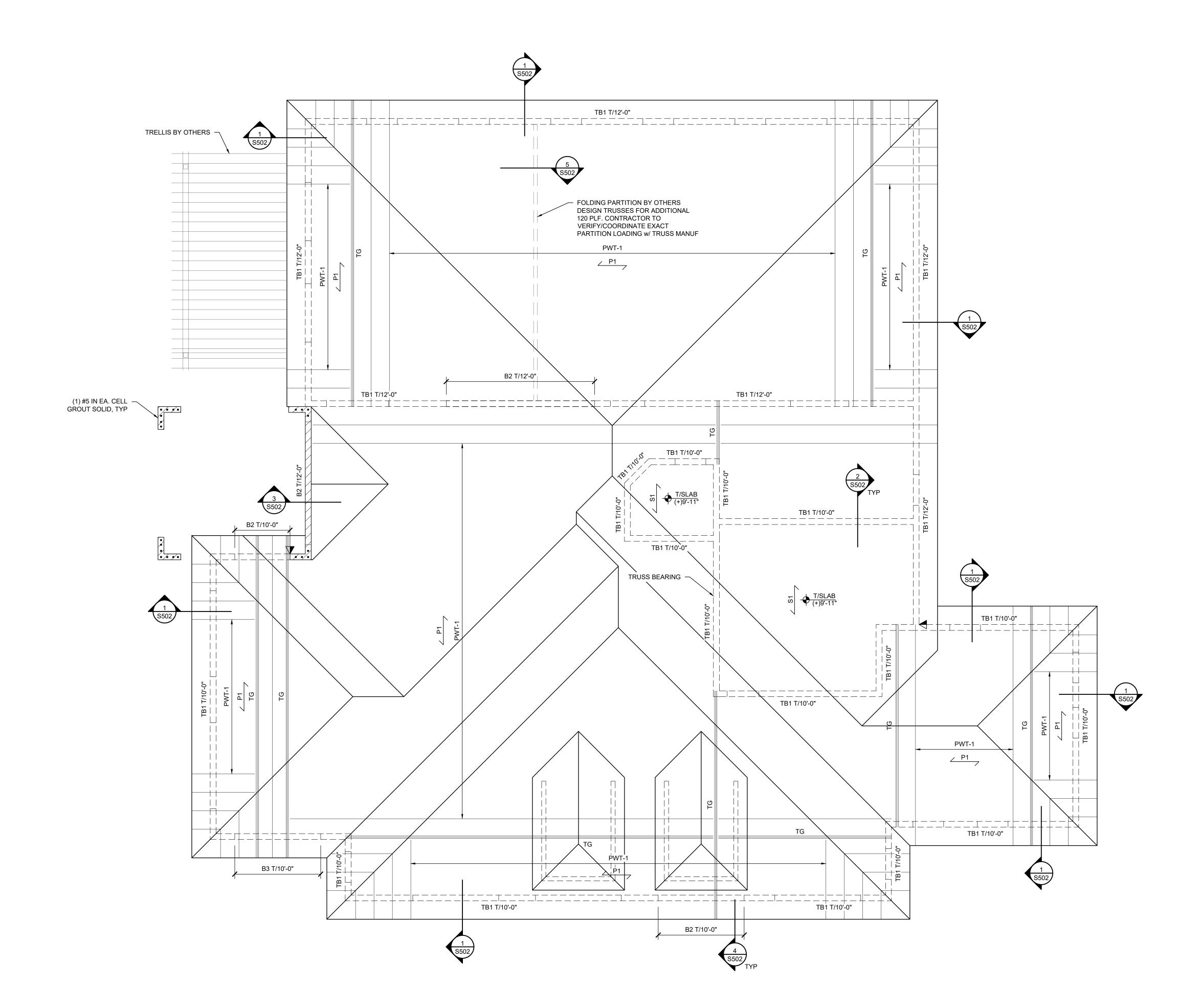
ENGINEERING

AFFIXED SEAL WITH ENGINEER'S SIGNATURE CERTIFIES THAT THE STRUCTURAL COMPONENTS OF THIS PLAN ARE IN COMPLIANCE TO THE FLORIDA BUILDING CODES 8TH EDITION (2023). RESISTANCE TO WIND PRESSURE IS IN COMPLIANCE WITH SECTION 1609. SIGNED/SEALED PLANS ARE VALID FOR ONE YEAR ONLY FROM DATE SIGNED.

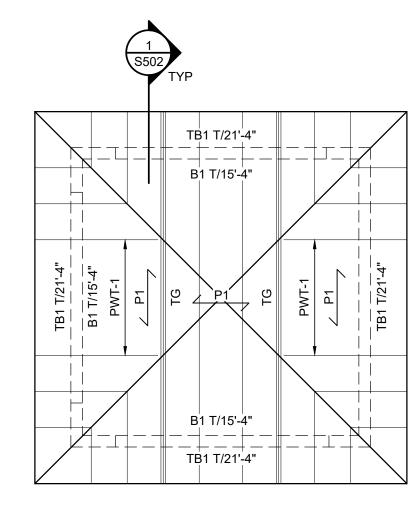
COREY M. SOLICH - PE 96895 REGISTRY # 35292



08 - 08 - 2024









WOOD TRUSS UPLIFT CONNECTOR TABLE				
UPLIFT ANCHOR	ALLOWABLE UPLIFT (lb)	ADDITIONAL INFORMATION / REMARKS		
WOOD TO WOOD				
HTS20	1,415	-		
(2) HTS20	2,710	-		
MSTC40	4,735	-		
SPH(4/6/8)	1,200	-		
WOOD TO CONCRE	ETE			
HETA20	1,810	TYPICAL FOR 1 OR MORE PLY TRUSSES		
(2) HETA20	1,920	TYPICAL FOR 1 PLY TRUSSES		
(2) HETA20	2,560	TYPICAL FOR 2 OR MORE PLY TRUSSES		
HTSM20	1,110	POST - INSTALLED		
MSTCM40	2,500	REFER TO MFR. FOR MIN. EDGE DISTANCE		
MGT	3,275	TYP FOR 1 PLY, USE (22) 10d x 1 1/2" NAILS		
MGT	4,365	TYPICAL FOR 2 OR MORE PLY TRUSSES		
HGT-2	10,345	-		
FGTR	4,725	FGTRHL/R (3,635 MAX. UPLIFT)		
(2) FGTR	8,885	(2) FGTRHL/R (6,800 MAX. UPLIFT)		
HTT4	4,235	-		
HTT5	5,090	-		
HDQ8-SDS	7,630	3 1/2" x 3 1/2" (MIN. WOOD MEMBER SIZE)		
HHDQ11-SDS	11,810	3 1/2" x 5 1/2" (MIN. WOOD MEMBER SIZE)		
HHDQ14-SDS	13,710	5 1/2" x 5 1/2" (MIN. WOOD MEMBER SIZE)		

- 1. ALL UPLIFT CONNECTORS SHALL BE MANUFACTURED BY SIMPSON STRONG-TIE (OR APPROVED EQUAL). INSTALL MAXIMUM SIZE AND NUMBER OF FASTENERS PER LATEST MANUFACTURER SPECIFICATIONS.
- 2. AT MISSED STRAPS, PROVIDE HTSM20 WITH 1 1/2" MINIMUM EDGE DISTANCE. FOR UPLIFTS UP TO 4,365 LBS, PROVIDE MGT
- WITH 5/8" DIA ATR EMBED 12" MINIMUM.

 3. IF TRUSS BEARS ON WALL BETWEEN CUT AND ADJACENT WALL STUD, THEN ADD WALL STUD WITHIN 1" OF CUT.

 4. FOR TWO EMBEDDED STRAPS ON A SINGLE PLY TRUSS, ANCHORS MUST BE OFFSET FROM EACH OTHER A MINIMUM OF
- 3/4" TO AVOID CONFLICTS. 5. FOR SINGLE PLY, SCABS TO MATCH 2X SYP #2 WITH 10D COMMON NAILS @ 4" O.C. STAGGERED. FOR TRUSS HEEL HEIGHTS OF 36" OR MORE, PROVIDE A 36" VERTICAL SCAB. FOR HEEL HEIGHTS LESS THAN 36", PROVIDE 36" TOP AND
- BOTTOM CHORD SCABS AND A FULL HEIGHT VERTICAL SCAB IF APPLICABLE. 6. SEE GENERAL STRUCTURAL NOTES FOR ADHESIVE ANCHORING SOLUTIONS. MINIMUM EMBED SHALL BE 12" ON ALL POST INSTALLED UPLIFT CONNECTORS. ADD 4" TO EMBED FOR LINTEL BEAMS.

CONCRETE BEAM SCHEDULE

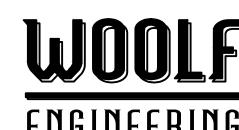
CONCRETE BEAM SCHEDULE						
TYPE	1 VVII) I H	DEPTH	REINFORCING			STIRRUP SPACING
MARK			TOP	MID	BOT	STIRRUP SPACIN
TB1	8"	1'-4"	(2) #5	ı	(2) #5	#3 @ 48" O.C.
B1	8"	1'-4"	(2) #5	ı	(2) #5	#3 @ 6" O.C.
B2	8"	2'-0"	(2) #5	(2) #5	(2) #5	#3 @ 6" O.C.
В3	8"	2'-0"	(2) #6	(2) #5	(2) #6	#3 @ 6" O.C.

NOTES:

1. PROVIDE PRECAST LINTEL BY CASTCRETE OR APPROVED EQUAL, TYPICAL UNLESS NOTED OTHERWISE, BELOW BEAMS AND ABOVE MASONRY OPENINGS AS PER LINTEL SCHEDULE.

2. PROVIDE #3 STIRRUPS @ 48" OC IN CONCRETE BEAMS, TYPICAL UNLESS NOTED OTHERWISE. HIGH ROOF FRAMING PLAN NOTES:

- 1. SEE SHEET S001 FOR GENERAL STRUCTURAL NOTES.
- 2. DO NOT SCALE DRAWINGS. VERIFY / COORDINATE ALL DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL DRAWINGS BEFORE COMMENCING CONSTRUCTION. NOTIFY STRUCTURAL ENGINEER AND ARCHITECT OF ANY DISCREPANCIES BEFORE PROCEEDING WITH WORK.
- 3. CONTRACTOR TO VERIFY ALL EXISTING CONDITIONS, DIMENSIONS AND ELEVATIONS PRIOR TO CONSTRUCTION AND NOTIFY STRUCTURAL ENGINEER AND ARCHITECT OF ANY DISCREPANCIES BEFORE PROCEEDING WITH WORK.
- 4. TX'-X" INDICATES TOP OF BEAM ELEVATION.
- INDICATES CONCRETE BEAM TYPE, SEE SCHEDULE THIS
- 6. P1 INDICATES SPAN DIRECTION OF 5/8" PLYWOOD SHEATHING. ATTACH TO INTERMEDIATE SUPPORTS W/ 8d RINGSHANK NAILS AT 6" OC AND EDGES AT 4" OC.
- 7. PWT-1 INDICATES PRE-ENGINEERED WOOD TRUSSES @ 24" OC.
- INDICATES PRE-ENGINEERED WOOD TRUSS GIRDER.
- 9. ____ INDICATES CONCRETE BEAM OR WALL BELOW.



GENERAL CONTRACTOR TO VERIFY ALL DIMENSIONS, DETAILS, AND SPECIFICATIONS SHOWN. OMISSIONS AND/OR ERRORS SHALL BE BROUGHT TO THE ATTENTION OF WOOLF ENGINEERING BEFORE COMMENCEMENT OF THE PROJECT. CHANGES MADE TO THESE PLANS BY THE G.C. OR OTHERS ARE THE SOLE RESPONSIBILITY OF THE GENERAL CONTRACTOR. THE CONCEPTUAL DESIGN AND LAYOUT FOR THIS STRUCTURE HAS BEEN PROVIDED TO WOOLF ENGINEERING BY THE OWNER OR CLIENT AND ANY SIMILARITIES TO ANOTHER PLAN OR DESIGN IS THE TOTAL RESPONSIBILITY OF THE OWNER OR CLIENT.

AFFIXED SEAL WITH ENGINEER'S SIGNATURE CERTIFIES THAT THE STRUCTURAL COMPONENTS OF THIS PLAN ARE IN COMPLIANCE TO THE FLORIDA BUILDING CODES 8TH EDITION (2023). RESISTANCE TO WIND PRESSURE IS IN COMPLIANCE WITH SECTION 1609. SIGNED/SEALED PLANS ARE VALID FOR ONE YEAR ONLY FROM DATE SECTION.

This document has been digitally signed using a SHA256 authentication code. Printed copies of this document are not considered signed and sealed

20020 VETERANS BLVD, #20 SECTION SIGNED. PORT CHARLOTTE, FL" PH: (941) 764-0200 info@woolfeng.com

COREY M. SOLICH - PE 96895 REGISTRY # 35292

08 - 08 - 2024

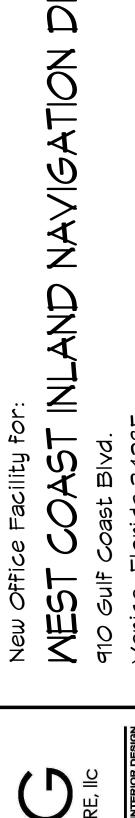
4161 Tamiami Trail #501 Port Charlotte, Florida 33952 Ph. (941) 639-2450 Fax (941) 639-2438

3820 Colonial Blvd. #100 Fort Myers, Florida 33966 Ph. (239) 277-0554 Fax (239) 277-0741

www.adgarchitecture.com

PROJECT NO. 2024-J-104









www.adgarchitecture.cor

08 - 08 - 2024

GENERAL CONTRACTOR. THE CONCEPTUAL DESIGN AND LAYOUT FOR THIS STRUCTURE HAS BEEN PROVIDED TO WOOLF ENGINEERING BY THE OWNER OR CLIENT AND ANY SIMILARITIES TO ANOTHER PLAN OR DESIGN IS THE TOTAL RESPONSIBILITY OF THE OWNER OR CLIENT.

PROJECT NO. 2024-J-104

LINTEL IS TO BE FILLED AT SAME TIME AS BEAM POUR. 20020 VETERANS BLVD, #20 PORT CHARLOTTE, FL

CMU WALL TIE BEAM PER PLAN PER PLAN PER PLAN GROUT SOLID - GROUT SOLID NOMINAL PRECAST LINTEL NOMINAL PRECAST LINTEL - PRECAST LINTEL

- QUANTITY OF #5 REBAR AT NOMINAL WIDTH ----BOTTOM OF LINTEL CAVITY

8F16-1B/1T F= FILLED WITH GROUT ----QUANTITY OF #5 REBAR AT TOP

NOMINAL HEIGHT

OPENING WIDTH

6' < W≤ 10'

10' < W≤ 16'

1 TYPICAL PRECAST LINTEL SCHEDULE

CAST IN PLACE REINFORCED PLACE METAL LATH OR WIRE CONCRETE TIE BEAM OR SCREEN OVER CORES NOT TO BE FILLED (SHEET METAL AND BOND BEAM FELT PROHIBITED) HORZ REINF SEE GENERAL VERTICAL REINFORCING SEE PLAN-FOR SIZE AND LOCATION NOTES CENTERED IN CELL GROUT FILLED CELL PLACE AND CONSOLIDATE GROUT IN 4 FOOT LIFTS MORTAR ACROSS WEBS SET FIRST COURSE IN FULL MORTAR BEDDING ADJACENT TO FILLED CELLS CLEAN OUT OPENINGS AT GROUT FILLED CELL LOCATIONS FOR WALLS OVER 48" HIGH - FOUNDATION

9 TYPICAL MASONRY WALL CONSTRUCTION DETAIL NOT TO SCALE

5 TYPICAL CONCRETE BEAM BEARING DETAIL NOT TO SCALE

EXTEND ALL REINFORCING

TO FAR FACE

CONCRETE FOOTING

PER PLAN

1/2 STIRRUP SPACING

STIRRUP SPACING

PER SCHEDULE

DROP BEAM BEARING

DEPTH AS REQ'D TO

MAINTAIN COURSING

TYPICAL FOOTING CORNER DETAIL
NOT TO SCALE

CLASS B LAP

16" MIN.

(1) #5 EA. LAYER

2'-6"

2'-6"

(2) #5 VERTICALS AT

#5 BARS IN GROUT FILLED CELLS. (2) BARS AT STEP (MIN.) 6 TYPICAL STEPPED TIE BEAM DETAIL
NOT TO SCALE

PRECAST -

LINTEL

CONCRETE

OPENING

10 TYPICAL MASONRY WALL OPENING DETAIL NOT TO SCALE

CONCRETE TIE BEAM

MASONRY COURSES

VERTICAL REINFORCING

PLAN FOR ADDITIONAL

GROUT SILL SOLID, AT

MATCH TYPICAL WALL

BELOW OPENINGS

REINFORCING ABOVE AND

OPENINGS 4'-0" AND WIDER

REINFORCING REQUIREMENTS

REINFORCED

MASONRY WALL

PROVIDE (1) #5

AT EACH OPENING - SEE 里

AS REQUIRED

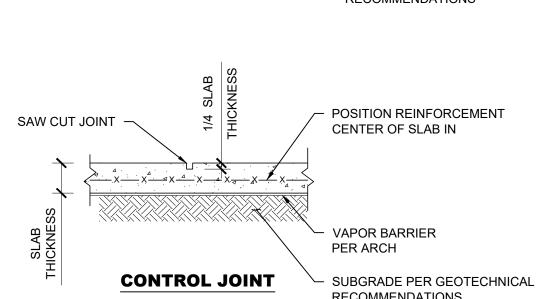
PER PLAN

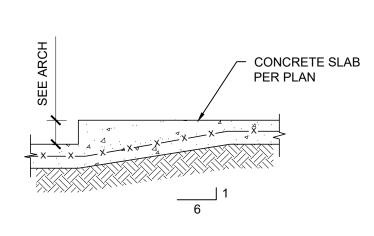
#5 BENT BAR REINFORCING PER PLAN CMU WALL PER PLAN

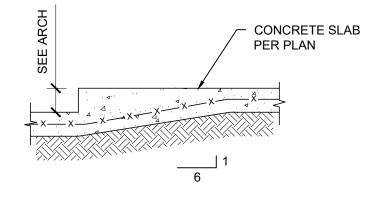
CONCRETE TIE BEAM PER PLAN

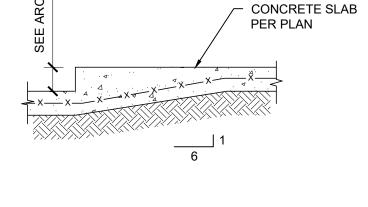
2 TYPICAL SLAB-ON-GRADE JOINT DETAIL NOT TO SCALE

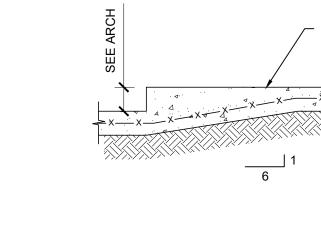
POSITION REINFORCEMENT SAW CUT JOINT CENTER OF SLAB IN VAPOR BARRIER **CONTROL JOINT** SUBGRADE PER GEOTECHNICAL RECOMMENDATIONS





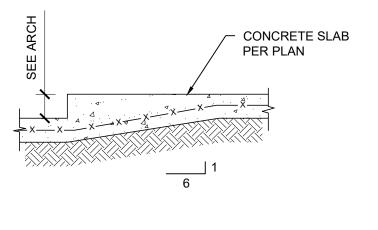






VERTICAL REINFORCEMENT ----

IN GROUTED CELLS



CONCRETE BEAM

CONCRETE BEAM

PER PLAN

USE SAME CORNER REINFORCING

FACTORY FABRICATED TEE 7
JOINT REINF. 3'-0" MIN LEGS

EVERY OTHER COURSE (TYP.)

MIN.

JOINT REINF.

CONTINUOUS HORIZONTAL

VERTICAL REINFORCEMENT -

IN GROUTED CELLS @

TYPICAL SPACING

AT OTHER ANGLED CORNERS

PER PLAN

(2) #5 CORNER

RÉINFORCING EA. LAYER

EXTEND ALL

REINFORCING

(1) #5 CORNER 2'-6"

REINFORCING EA. LAYER

EXTEND ALL

REINFORCING

TO FAR FACE

TYPICAL TIE BEAM CORNER DETAIL

HOOK TOP BARS

VERTICAL REINFORCEMENT

CONTINUOUS HORIZONTAL

CONTINUOUS HORIZONTAL

- CONCRETE MASONRY WALL

This document has been digitally signed using a

GENERAL CONTRACTOR TO VERIFY ALL DIMENSIONS, DETAILS, AND SPECIFICATIONS SHOWN. OMISSIONS AND/OR ERRORS SHALL BE BROUGHT TO THE ATTENTION OF WOOLF ENGINEERING BEFORE COMMENCEMENT OF THE PROJECT. CHANGES MADE TO THESE PLANS BY THE G.C. OR OTHERS ARE THE SOLE RESPONSIBILITY OF THE

AFFIXED SEAL WITH ENGINEER'S SIGNATURE CERTIFIES THAT THE STRUCTURAL COMPONENTS OF THIS PLAN ARE IN COMPLIANCE TO THE FLORIDA BUILDING CODES 8TH EDITION (2023). RESISTANCE TO WIND PRESSURE IS IN COMPLIANCE WITH SECTION 1609. SIGNED/SEALED PLANS ARE VALID FOR ONE YEAR ONLY FROM DATE

COREY M. SOLICH - PE 96895 REGISTRY # 35292

SHA256 authentication code. Printed copies of this document are not considered signed and sealed

JOINT REINF.

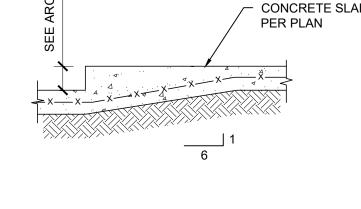
8 TYPICAL JOINT REINF. AT INTERSECTION DETAIL NOT TO SCALE

PH: (941) 764-0200 info@woolfeng.com IN GROUTED CELLS

JOINT REINF.

MORTAR JOINT

TO FAR FACE



3 TYPICAL SLAB RECESS/STEP DETAIL
NOT TO SCALE

FACTORY FABRICATED CORNER.

SECTION OF HORIZONTAL JOINT

CONTINUOUS HORIZONTAL

VERTICAL REINFORCEMENT IN

GROUTED CELLS @ TYPICAL SPACING

REINFORCEMENT

JOINT REINF.

MIN.

JOINT REINF.

TYPICAL JOINT REINF. AT CORNER DETAIL

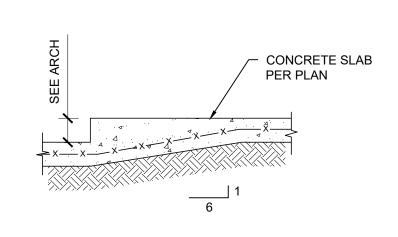
LINTEL SIZE 8F16-1B/1T, 12F16-1B/1T

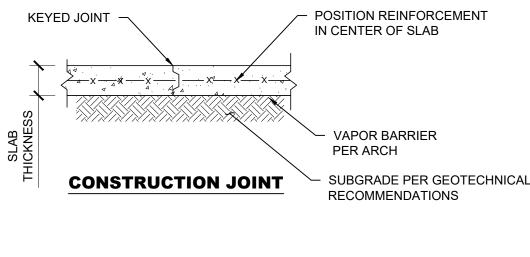
8F16-1B/2T, 12F16-1B/2T

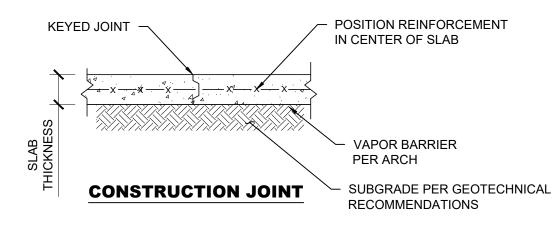
8F24-1B/2T, 12F24-1B/2T

CONTINUOUS HORIZONTAL

CONCRETE MASONRY WALL







4161 Tamiami Trail #501

Port Charlotte, Florida 33952 Ph. (941) 639-2450 Fax (941) 639-2438

3820 Colonial Blvd. #100 Fort Myers, Florida 33966 Ph. (239) 277-0554 Fax (239) 277-0741

AA-26002422

www.adgarchitecture.com

08 - 08 - 2024

CMU WALL PER PLAN HORIZ JOINT REINF PER NOTES START AT FIRST JOINT, TYP

-x-x-x-x-2x-2x DOWEL TO MATCH -WALL REINFORCING

ALL CELLS BELOW GRADE GROUTED SOLID DOWEL TO MATCH -WALL REINFORCING CONCRETE FOOTING PER PLAN

(1) #5 CONT.

__ #4 x 5'-0"@ 24" O.C.

SLAB-ON-GRADE PER PLAN



(1) #5 CONT.

FINISH GRADE PER SITE/CIVIL

DOWEL TO MATCH WALL REINFORCING

- #4 @ 16" O.C. 6" 3'-0"

PER PLAN

ALL CELLS BELOW

PER PLAN

GRADE GROUTED SOLID

- CONCRETE FOOTING

— SLAB-ON-GRADE

3 INTERIOR DOOR THRESHOLD DETAIL NOT TO SCALE

4 INTERIOR CMU FOUNDATION DETAIL
NOT TO SCALE

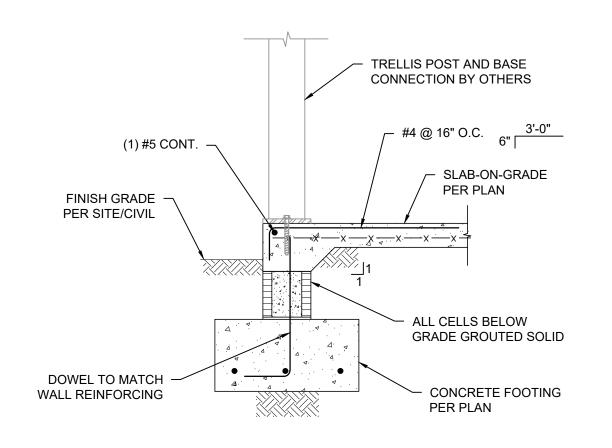
SLAB-ON-GRADE PER PLAN

ALL CELLS BELOW

PER PLAN

GRADE GROUTED SOLID

CONCRETE FOOTING



5 TRELLIS FOUNDATION DETAIL NOT TO SCALE

EXTERIOR CMU FOUNDATION DETAILNOT TO SCALE

SLAB-ON-GRADE PER PLAN

ALL CELLS BELOW

PER PLAN

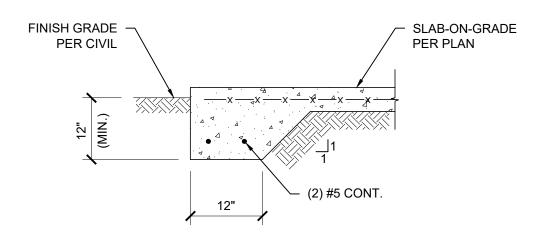
GRADE GROUTED SOLID

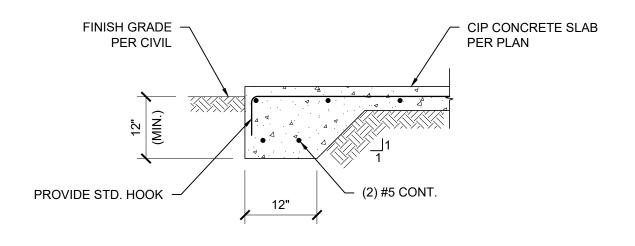
CONCRETE FOOTING

CMU WALL PER PLAN

FINISH GRADE -PER SITE/CIVIL

DOWEL TO MATCH -WALL REINFORCING

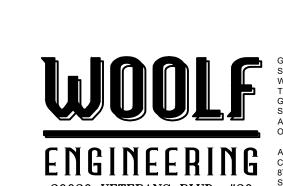




NOTE: COORDINATE LOCATION OF EQUIPMENT BOLTS AND PAD WITH EQUIPMENT SUPPLIER'S DRAWING. SEE ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR LOCATION AND SIZE OF EQUIPMENT PADS.







This document has been digitally signed using a SHA256 authentication code. Printed copies of this document are not considered signed and sealed

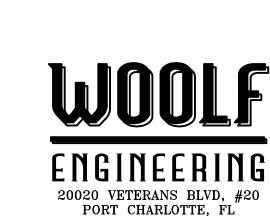
GENERAL CONTRACTOR TO VERIFY ALL DIMENSIONS, DETAILS, AND SPECIFICATIONS GENERAL CONTRACTOR TO VERIFY ALL DIMENSIONS, DETAILS, AND SPECIFICATIONS SHOWN. OMISSIONS AND/OR ERRORS SHALL BE BROUGHT TO THE ATTENTION OF WOOLF ENGINEERING BEFORE COMMENCEMENT OF THE PROJECT. CHANGES MADE TO THESE PLANS BY THE G.C. OR OTHERS ARE THE SOLE RESPONSIBILITY OF THE GENERAL CONTRACTOR. THE CONCEPTUAL DESIGN AND LAYOUT FOR THIS STRUCTURE HAS BEEN PROVIDED TO WOOLF ENGINEERING BY THE OWNER OR CLIENT AND ANY SIMILARITIES TO ANOTHER PLAN OR DESIGN IS THE TOTAL RESPONSIBILITY OF THE OWNER OR CLIENT.

ENGINEERING

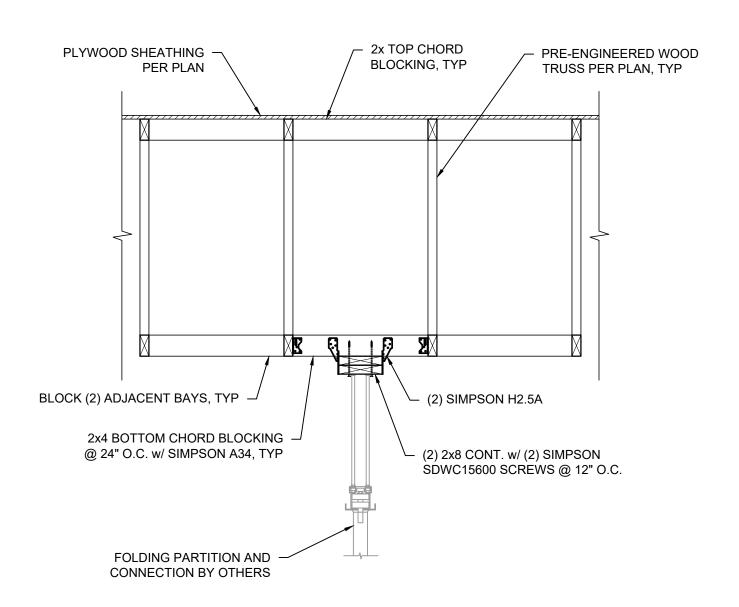
20020 VETERANS BLVD, #20
PORT CHARLOTTE, FL

AFFIXED SI
COMPONENT
SH EDITION
SECTION 16
SIGNED. PH: (941) 764-0200 ${f info@woolfeng.com}$

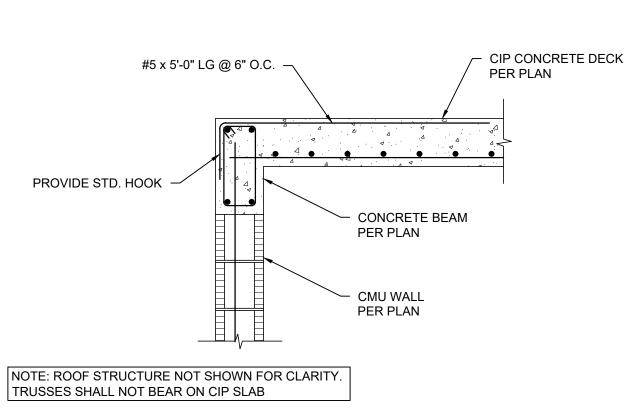
AFFIXED SEAL WITH ENGINEER'S SIGNATURE CERTIFIES THAT THE STRUCTURAL COMPONENTS OF THIS PLAN ARE IN COMPLIANCE TO THE FLORIDA BUILDING CODES 8TH EDITION (2023). RESISTANCE TO WIND PRESSURE IS IN COMPLIANCE WITH SECTION 1609. SIGNED/SEALED PLANS ARE VALID FOR ONE YEAR ONLY FROM DATE COREY M. SOLICH - PE 96895 REGISTRY # 35292 PROJECT NO. 2024-J-104



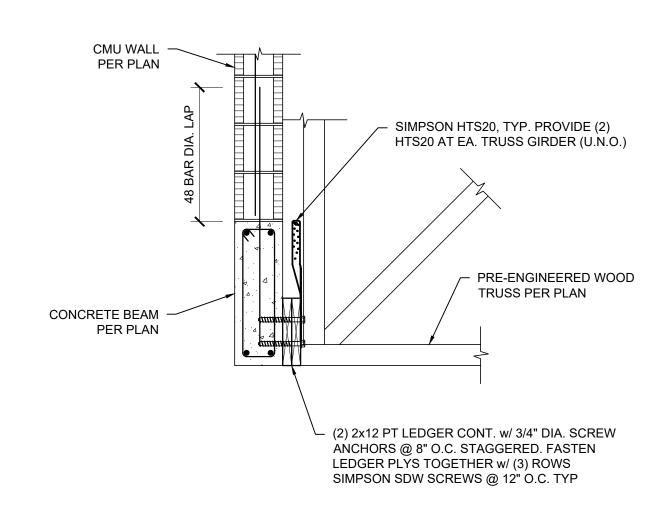
TRUSS BRG AT CMU WALL DETAIL NOT TO SCALE



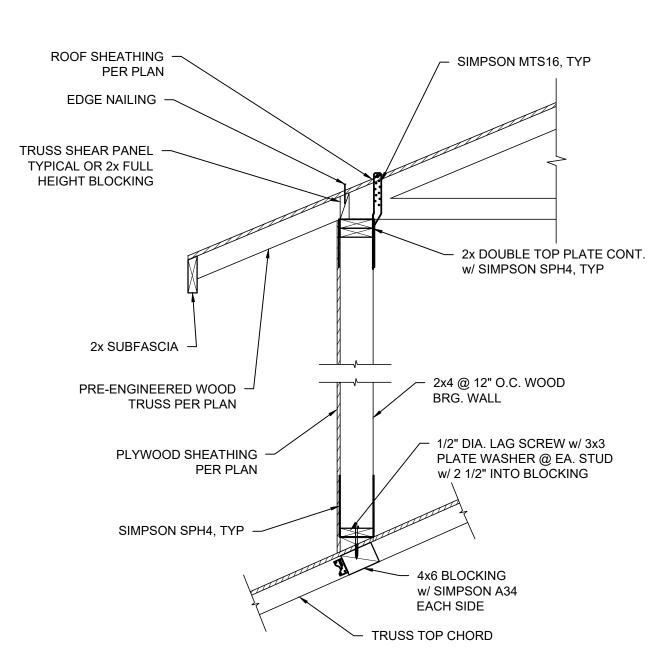
5 FOLDING PARTITION AT TRUSS DETAIL
NOT TO SCALE



2 CONC. DECK BRG. AT CMU WALL DETAIL
NOT TO SCALE

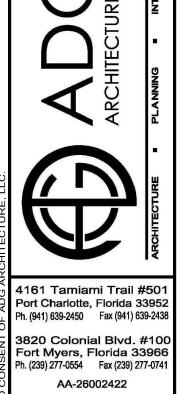


3 TRUSS BRG AT CONC. BEAM DETAIL
NOT TO SCALE



ROOF TRUSS DORMER DETAIL
NOT TO SCALE

MEST COAST IN 910 Gulf Coast Blvd.





mit 9-24-2024

issued for Permit 9-24

This document has been digitally signed using a SHA256 authentication code. Printed copies of this document are not considered signed and sealed

GENERAL CONTRACTOR TO VERIFY ALL DIMENSIONS, DETAILS, AND SPECIFICATIONS SHOWN. OMISSIONS AND/OR ERRORS SHALL BE BROUGHT TO THE ATTENTION OF WOOLF ENGINEERING BEFORE COMMENCEMENT OF THE PROJECT. CHANGES MADE TO THESE PLANS BY THE G.C. OR OTHERS ARE THE SOLE RESPONSIBILITY OF THE GENERAL CONTRACTOR. THE CONCEPTUAL DESIGN AND LAYOUT FOR THIS STRUCTURE HAS BEEN PROVIDED TO WOOLF ENGINEERING BY THE OWNER OR CLIENT AND ANY SIMILARITIES TO ANOTHER PLAN OR DESIGN IS THE TOTAL RESPONSIBILITY OF THE OWNER OR CLIENT.

PORT CHARLOTTE, FL" PH: (941) 764-0200 info@woolfeng.com

AFFIXED SEAL WITH ENGINEER'S SIGNATURE CERTIFIES THAT THE STRUCTURAL COMPONENTS OF THIS PLAN ARE IN COMPLIANCE TO THE FLORIDA BUILDING CODES 8TH EDITION (2023). RESISTANCE TO WIND PRESSURE IS IN COMPLIANCE WITH SECTION 1609. SIGNED/SEALED PLANS ARE VALID FOR ONE YEAR ONLY FROM DATE SIGNED.

S502

08 - 08 - 2024

COREY M. SOLICH - PE 96895 REGISTRY # 35292

COREY M. SOLICH - PE 96895 REGISTRY # 35292

COREY M. SOLICH - PE 96895 REGISTRY # 35292

COREY M. SOLICH - PE 96895 REGISTRY # 35292